





Introduction to Traffic Loads

Definition

Traffic loads refer to the forces applied to pavement by vehicles in motion.



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Traffic Loads

Impact

- Pavement deterioration is caused by the interacting damaging effects of traffic and the environment.
- Traffic loads, primarily those from heavy trucks, cause stresses/strains in pavement structures, whose effects accumulate over time, resulting in pavement deterioration, Such as rutting







Traffic Loads

Quantification criteria

Traffic Loads

Quantification criteria

Pavement Damage depends on weight distribution

Truck traffic loads and their impact on pavements are Quantify using :

- 1. Vehicle/axle speed
- 2. Number of truck axles
- 3. Configuration of these axles
- 4. Their load magnitude
- 5. Tire inflation pressure



Vehicle/axle speed

Static loading

 The two basic types of loads are static and dynamic

Static loading

- implies a sustained loading of the structure over a period of time.
- Generally, static loads are slowly applied such that no shock or vibration is generated in the structure.
- Once applied, the static load may remain in place or be removed slowly.
- Loads that remain in place for an extended period of time are called sustained (dead) loads.



















Tire Configuration

Single Tire

■ Typical Load per Tire: 20 - 50 kN



Tire Configuration

Dual Tire

■ Typical Load per Tire: 40 - 100 kN



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Tire Configuration

Wide Base Tire

- Description: Extra wide tire designed to replace duals for weight savings.
- Typical Load per Tire: 60
 100 kN











What do you think?

Axle Tire Configuration







	IG AND WEIGHING	12
 Definition Weigh-In-Motion (WIM) is a technology used to measure the weight of vehicles as they pass over a sensor at highway speeds, without requiring them to stop. 		
Main Components:		
1.Sensors : Detect and measure the load of vehicles.		RECORD:
2.Data Processing Unit: Analyzes the collected data.	AXLE 1- AXLE 2- AXLE 3-	7.6 t 12.8 t 7.3 t
3.Communication System : Transmits the data to central servers	AXLE 4 - AXLE 5 - LP FRONT SPEED	7.2 t 7.4 t BBB 716 90 km/h
	LENGTH WIDTH HEIGHT	14.9 m 2.45 m 3.2 m
	LP BACK	BBB 716







FHW	A vehi	cle Weigh	class	5
	FHWA Class Type	Class Definition	Axle Group	Example truck configurations
	5	Two-axle, six-tire, single-unit trucks	1	
	6	Three-axle single- unit trucks	1 and 2	
	7	Four or more axle single-unit trucks	1, 3 and 4	6
	8	Four or fewer axle single-trailer trucks	1 and 2	
	9	Five-axle single- trailer trucks	1 and 2	
	10	Six or more axle single-trailer trucks	1, 2, 7 and 8	
	11	Five or fewer axle multi-trailer trucks	1	
	12	Six-axle multi- trailer trucks	1 and 2	
	13	Seven or more axle multi-trailer trucks	1, 2, 3, 4, 5, 7and 8	5 000 00 00 5 00 000 00 00 5 00 000 0 000 5 0000 00 00 5 00 000 0 00 5 00 0000 5 0000 00 00 5 00 00 00 5 00 00000 5 0000 00 00 5 00 00 00 5 000 00000





Load distribution



Axle Load distribution Tridem axle carries more load with an average weight of when compared with tandem and single 10.5 tonnes axle configurations, ■ Tandem axle carries more <u>о</u> <u>п</u> О load than single axle. Truck weight Truck & trailer 9.5 tonnes 4.5 tonnes 3.0 tonnes 2.0 tonnes 4.0 tonnes 6.5 tonnes 10.5 tonnes Container 20.0 tonnes

V	ehicle w	eight distributi	on.			10.5 1	tonnes	
_ /	Axle loads	Vehicle weight					-0.0	
	Axie Configura	tion.						Truck weight
•	Front axle: Sing	gle tire (1 tire total)		Truck & trailer	4.5 tonnes	3.0 tonnes	2.0 tonnes	9.5 tonnes
•	Second axle: Si	ingle axle with double tires (2	2	Container		4.0 tonnes	6.5 tonnes	10.5 tonnes
	tires total)							20.0 tonnes
•	Tandem axles: tires total)	Dual tires (4 tires per axle, 8						
		Axle Type	Axle L	oad	Number of	Load per	Tire	
			(Tonne	es)	Tires	(Tones)		
		Front Axle (Single Tire)	4.5		1	4.5		
		Second Axle (Double Tires)	3.0		2	1.5		
		Tandem Axles (Dual Tires)	12.5		8	1.5625		
								30













- The collected traffic data must be summarized in a format that is suitable for direct input into the pavement design process, ensuring accurate traffic loading estimates for long-term pavement performance analysis.
- Available Approaches:
- ESALs appraoch (AASHTO 1986/1993 Pavement Design Approach)
 Load spectra (NCHRP 1-37A Pavement Design Approach)















	TABLE '	7.1 Average Initia	ll Truck Factors (ESALs/Truck) b	y Vehicle	e Class
FHWA Vehicle Classifications	V	EHICLE CLASS	IFICATION	ES	AL's
2 valini, d'a ratin 2 valini,	Line # in DARWin [®] 3.01	FHWA Class	Corresponding Department Description	Rigid	Flexible
2 zolis, 6 triss (suid nair fres), single unit 3 zolis, 6 triss (suid nair fres), single unit 5 zolis, 6 triss (suid nair fres), single trailer 4 or non aulas, single unit	1	1	Motorcycle	0*	0*
0. Single Trailer 5-Atle Trucks 10. Single Trailer 6 or More-Atle Trucks	2	2	Passenger Cars	0*	0*
5 axles, single trailer	3	3	SUV/Pick-up	0*	0*
	4	4	BUS Factor	0.24	0.24
	5	5	2-axle, 6-tire	0.24	0.24
11. Multi-Trailer & Sor Less-Ade Tracks 5 or less axles, multiple trailers 6 axles, multiple trailers	6	6	3-axle, single unit	1.15	0.82
	7	7	4-axle, single unit	7.00	4.50
13. Multi-Trailer 7 or More-Axie Trucks 7 or more axies, multiple trailers	8	8	3-axle, single trailer	0.60	0.44
	9	9	3-axle, multiple axle trailer	1.59	1.00
	10	10	6-axle, single trailer	1.42	0.75
	11	11	5-axle, multiple trailer	2.40	2.33
	12	12	6-axle, multiple trailer	1.42	1.28
	13	13	7-axle, multiple trailer	1.42	1.28
https://www.permdat.ga.gov/ProjectAnsPrograms/PosterBiondesRoadway/Documents/ESALBASEDX20000T5.pdf	*Note: Because the 18-kip ESAL percent of the AI 100.00%. If ther they should be gr	motorcycles, passeng s they are considered DT in this class must l e are any vehicles tha ouped with the motor	er cars, and SUV/Pick-up trucks do not sign negligible and an ESAL/truck factor of 0 is ue input into DARWin because the Total Pe t are not large enough to be classified in any cycle percentage.	ificantly co assigned. rcentage m 7 of the abo	ontribute to However, the ust equal ve classes,







Pavement Response to Tra

Flexible Pavements

- Load is distributed layer by layer, starting from the surface and spreading through the granular layers below.
- The stress diminishes with depth, as each layer contributes to spreading the load over a larger area.
- Key Features:
 - Load distribution depends on the characteristics of each layer, with the subgrade being a critical factor.
 - Flexible pavements exhibit higher deflection under load, as the layers work together to distribute the load.





Pavement Response to Traffic Loads

Rigid Pavements

- Load is distributed primarily by the concrete slab itself.
- The slab's stiffness allows it to spread loads over a wide area, minimizing stress on the layers below.
- Key Features:
 - The concrete slab carries most of the load, with little reliance on the underlying layers.
 - Rigid pavements exhibit less deflection under load, offering greater structural resilience





Asphalt Mixture design

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20-Year Design ESALs ^a (in millions)	Coarse Aggregate Angularity (Percent), minimum ^c AASHTO T 335 (CAA)		Uncompacted Fine Aggre (Percen AASH	d Void Content of gate Angularity t), minimum ITO T 304 (FAA)	Sand Equivalent (Percent), minimum AASHTO T 176	Flat and Elongated ^e (Percent), maximum ASTM D4791	
•	\leq 100 mm ^f	> 100 mm ^f	≤100 mm	> 100 mm ^f	(SE)	(F&E)	
< 0.3	55/-	-/-	_d	-	40		
0.3 to < 3	75/-	50/-	40 ^e	40	40	10	
3 to < 10	85/80 ^b	60/-	45	40	45	10	
10 to < 30	95/90	80/75	45	40	45	10	
	100/100	100/100	15	15	50	10	

NOTES:

a Design ESALs are the anticipated project traffic level expected on the design lane over a 20-year period. Regardless of the actual design life of the roadway, determine the design ESALs for 20 years and choose the appropriate NI levels

Marshall Method Criteria ¹	Light Traffic ³ Surface & Base		Medium Surface	Traffic ³ & Base	Heavy Traffic ³ Surface & Base		
	Min	Max	Min	Max	Min	Max	
Compaction, number of blows each end of specimen	35		50		75		
Stability², N (lb.)	3336 (750)	-	5338 (1200)	÷	8006 (1800)	-	
Flow ^{2,4,5} , 0.25 mm (0.01 in.)	8	18	8	16	8	14	
Percent Air Voids ⁷	3	5	3	5	3	5	
Percent Voids in Mineral Aggregate (VMA)⁵	See Table 7.3						
Percent Voids Filled With Asphalt (VFA)	70	80	65	78	65	75	
 All criteria, not just stability va Hot mix asphalt bases that do n criteria when tested at 38°C (10 applies only to regions having a United States. A different lower Traffic classifications Light Traffic conditions resu Medium Traffic conditions r 	ue alone, must ot meet these c 0°F) and are p range of climati test temperatur lting in a 20-yee esulting in a 20	t be considere riteria when tes laced 100 mm ic conditions si re may be cons ear Design ESA 0-year Design	d in designing a sted at 60°C (14 (4 inches) or mo milar to those p idered in regions L < 10 ⁴ ESAL between 1	n asphalt pav 40°F) are satis ore below the s revailing throu s having more 0 ⁴ and 10 ⁶	ing mix. factory if they mo surface. This reco Ighout most of th extreme climatic	eet the ommendation ne conditions.	