

Pavement Materials & Design

Aggregates Blending

Introduction

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Aggregate Production



<https://www.youtube.com/watch?v=iChSAXB16aw>

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Aggregate Production



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Aggregate classifications by size

Local classification



العدسية



الفولية



الجوزية



رمل



الناعمة (السمسية)



الحمسية



Image source: <https://www.slideserve.com/avei/4803086>

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Aggregates

1.2.2.2 Properties

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Properties of Aggregates

1. Particle size and gradation.
2. Hardness or resistance to wear
3. Durability or resistance to weathering.
4. Chemical stability
5. Particle shape and surface texture.
6. Freedom from deleterious particles or substances.
7. Specific gravity & absorption.

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Particle size distribution (gradation)



❑ Gradation

- **Blend of particle sizes in the mix.**
- It is *the most important* property of an aggregate

❑ Gradation affects on the following properties of asphalt mixes

1. Stiffness
2. Stability
3. Durability
4. Permeability
5. Workability
6. Fatigue resistance
7. Frictional resistance
8. Resistance to moisture damage
9. Economy of pavement structure.

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Particle size distribution (gradation)

❑ Gradation is evaluated by passing the aggregates through a series of sieves

- Called "Sieve analysis"

❑ Sieve analysis:

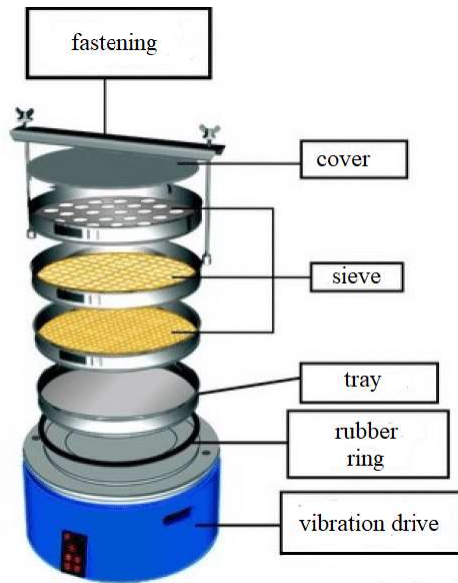
- Determination of *particle size distribution of fine and coarse aggregates by sieving,*
- Expressed as %
- Grain size analysis data are plotted on aggregate gradation chart



<https://civiconcepts.com/blog/sieve-analysis>

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Aggregate Mechanical Sieve



https://www.researchgate.net/publication/349246117_Mathematical_Modeling_of_Coal_Dust_Screening_by_Means_of_Sieve_Analysis_and_Coal_Dust_Combustion_Based_on_New_Methods_of_Piece-Linear_Function_Approximation/figures/fig1&utm_source=google&utm_medium=organic

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Aggregate Mechanical Sieve



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Sieve Analysis example



Example -1

- A sieve analysis test was performed on a sample of fine aggregate and produced the following results

Sieve, mm	4.75	2.36	2.00	1.18	0.60	0.30	0.15	0.075	pan
Amount retained, g	0	33.2	56.9	83.1	151.4	40.4	72.0	58.3	15.6

- Calculate the percent passing each sieve
- Draw a 0.45 power gradation chart with the use of a spreadsheet program.

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Example 1 – Solution



Percent passing each sieve

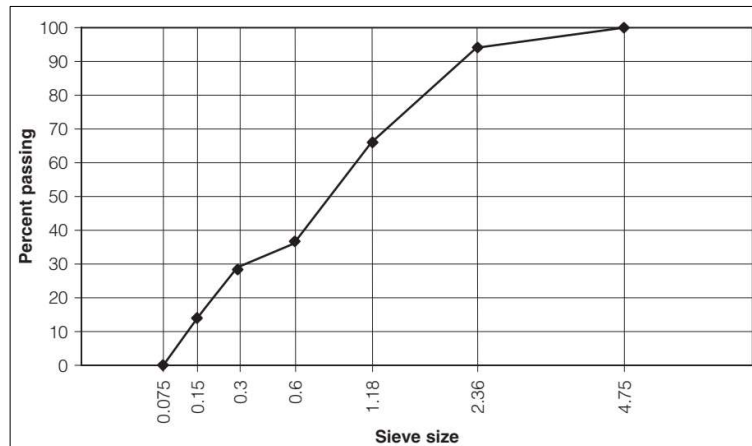
Sieve size	Amount Retained, g (a)	Cumulative Amount Retained, g (b)	Cumulative Percent Retained (c) = (b) × 100/Total	Percent Passing* (d) = 100 – (c)
4.75 mm (No. 4)	0	0	0	100
2.36 mm (No. 8)	33.2	33.2	6	94
2.00 mm (No. 10)	56.9	90.1	18	82
1.18 mm (No. 16)	83.1	173.2	34	66
0.60 mm (No. 30)	151.4	324.6	64	36
0.30 mm (No. 50)	40.4	365.0	71	29
0.15 mm (No. 100)	72.0	437.0	86	14
0.075 mm (No. 200)	58.3	495.3	96.9	3.1
Pan	15.6	510.9	100	
Total	510.9			

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Example 1 – Solution

Draw a 0.45 power gradation chart

1	2	3
Sieve Size (mm)	Sieve to the 0.45 Power	Percent Passing
4.75	2.02	100
2.36	1.47	94
2	1.37	82
1.18	1.08	66
0.6	0.79	36
0.3	0.58	29
0.15	0.43	14
0.075	0.31	3.1



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Aggregate

Aggregate classifications- by size based on

□ ASTM standard

➤ Coarse aggregate:

- ❖ Aggregate **retained** on Sieve **No. 4** (4.75 mm)

➤ Fine aggregate:

- ❖ Aggregate **passing** Sieve **No. 4** (4.75 mm) and retained on Sieve **No. 200** (0.075 mm)

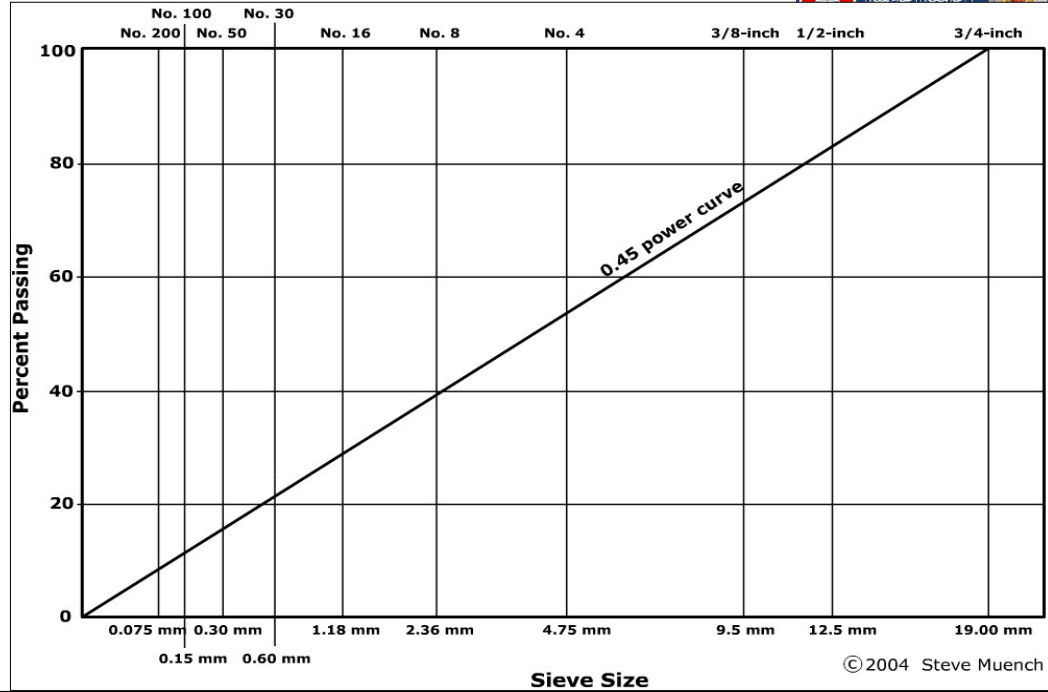
➤ Mineral fillers/dust/fines:

- ❖ Aggregate **passing** Sieve **No. 200** (0.075 mm)



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Aggregate classifications by size



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Maximum Aggregate Size

❑ Two parameters are used to represent the maximum aggregate size

1. Nominal Maximum Aggregate Size (NMAS)

❖ is the smallest sieve that retains some of the aggregate particles but generally not more than 10 percent by weight (according to ASTM standard)

2. Maximum aggregate size

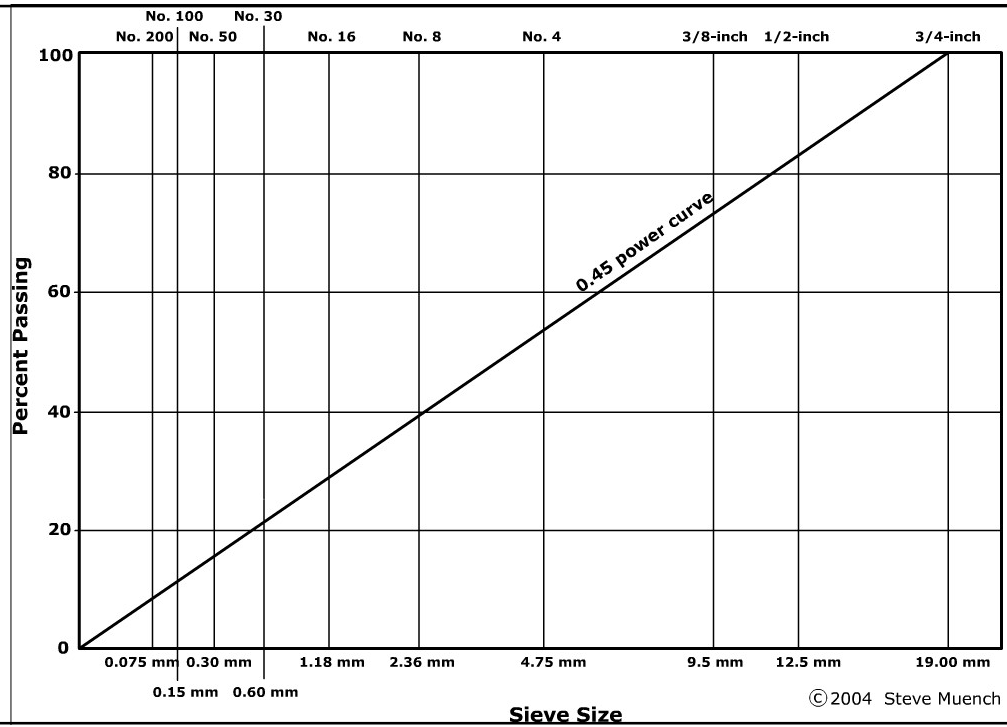
❖ The smallest sieve through which 100 percent of the aggregate sample particles pass (or retained 0) (according to ASTM standard)

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Aggregate gradation

Maximum density line

http://www.pavementinteractive.org/wp-content/uploads/2011/07/Gradation_terms.swf



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Aggregate

Typical Gradations

Dense or well-graded.

- Refers to a gradation that is near **maximum density**.
- The **most common HMA mix designs in the U.S. tend to use dense graded aggregate.**

Properties

- Good interlock
- Low permeability

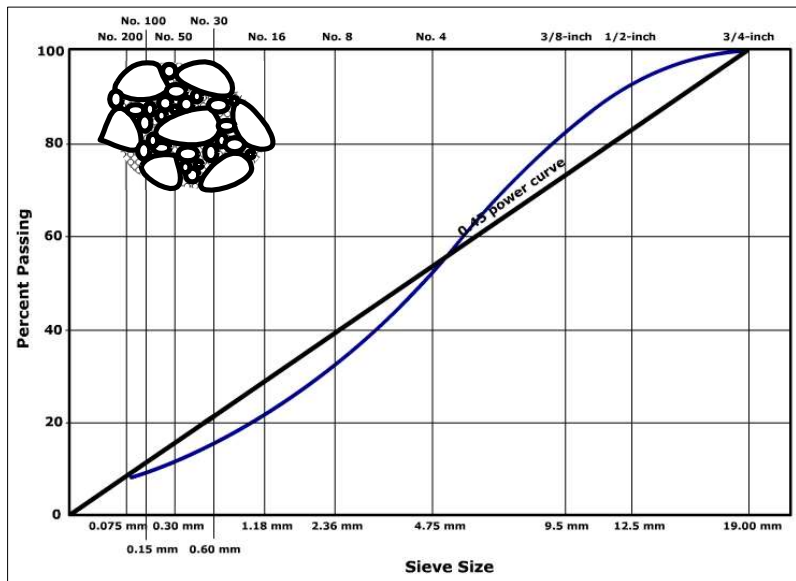
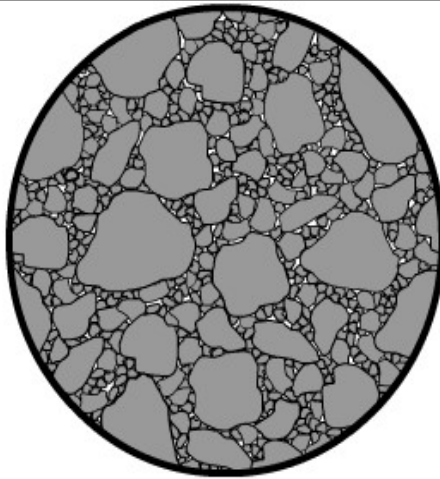


Image source: <https://pavementinteractive.org/reference-desk/materials/aggregate/gradation-and-size/>

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Dense-graded asphalt Mixtures



Dense graded HMA contains all sizes of aggregate particles. There are enough fine particles to effectively separate many of the coarse particles. Therefore, stress transmission through the HMA structure relies on both the coarse and fine particles. VMA is generally between 11 and 17%, air voids are generally near 4%, and asphalt binder content can range between 4.5 to 6%.

Aggregate

Typical Gradations

Uniformly graded

- Or single size or one size
- Refers to a gradation that contains most of the particles in a very narrow size range.
- The curve is steep and only occupies the narrow size range specified

Properties

- Few points of contact
- Poor interlock (shape dependent)
- High permeability

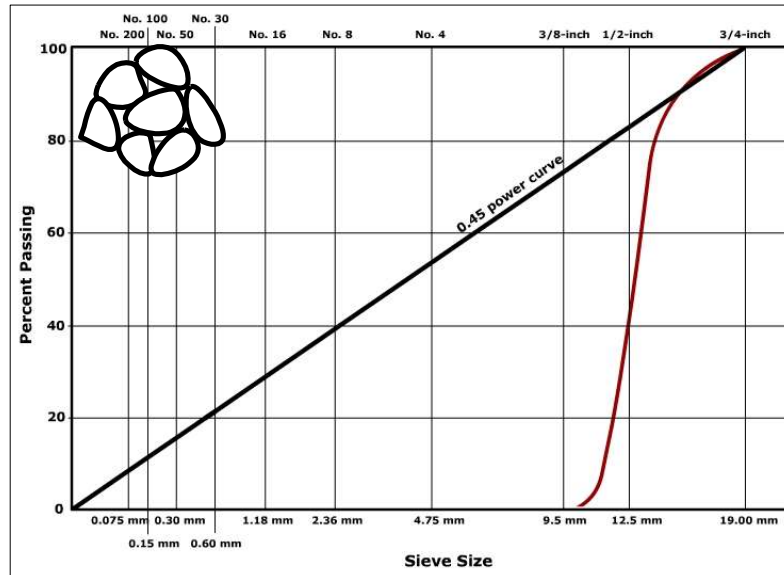


Image source: <https://pavementinteractive.org/reference-desk/materials/aggregate/gradation-and-size/>

Aggregate Typical Gradations



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Aggregate

Gradation specifications (limits)

- Gradation specifications is used to define **maximum and minimum** cumulative percentages of **material passing** each sieve

Sieve	Percent Passing
9.5 mm (3/8")	100
4.75 mm (No. 4)	95–100
2.36 mm (No. 8)	80–100
1.18 mm (No. 16)	50–85
0.60 mm (No. 30)	25–60
0.30 mm (No. 50)	10–30
0.15 mm (No. 100)	0–10

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Gradation Specifications



Representative Gradation Specifications for surface Course

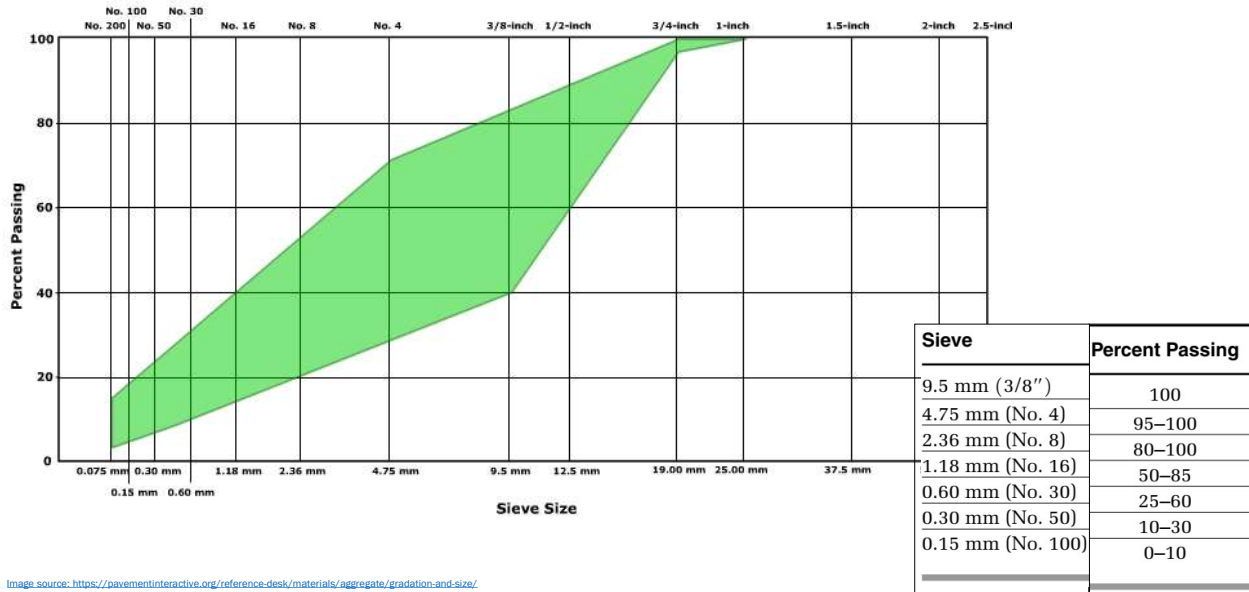


Image source: <https://pavementinteractive.org/reference-desk/materials/aggregate/gradation-and-size/>

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Gradation Specifications



Representative Gradation Specifications for Base Course

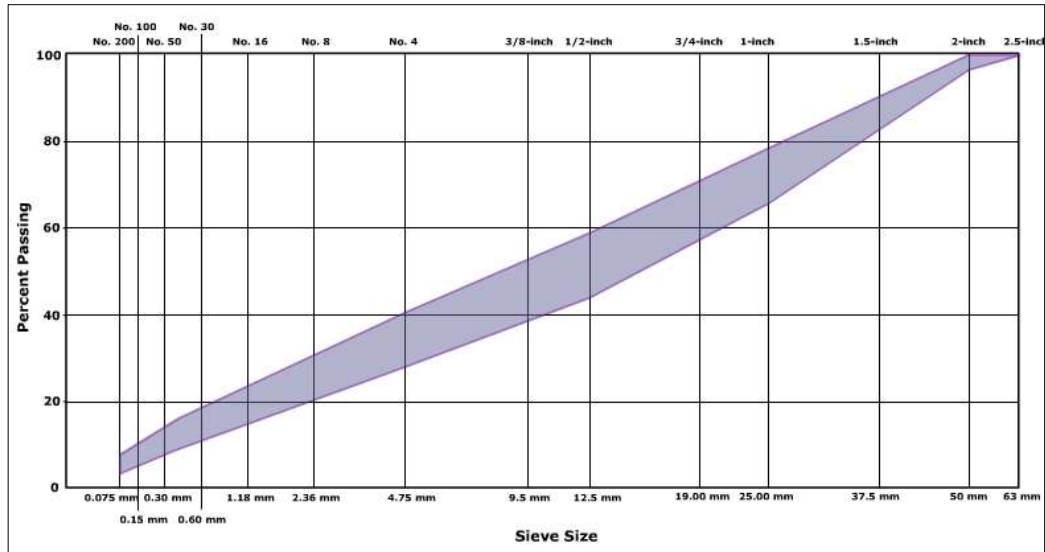


Image source: <https://pavementinteractive.org/reference-desk/materials/aggregate/gradation-and-size/>

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Gradation Specifications

Representative Gradation Specifications for Subbase Course

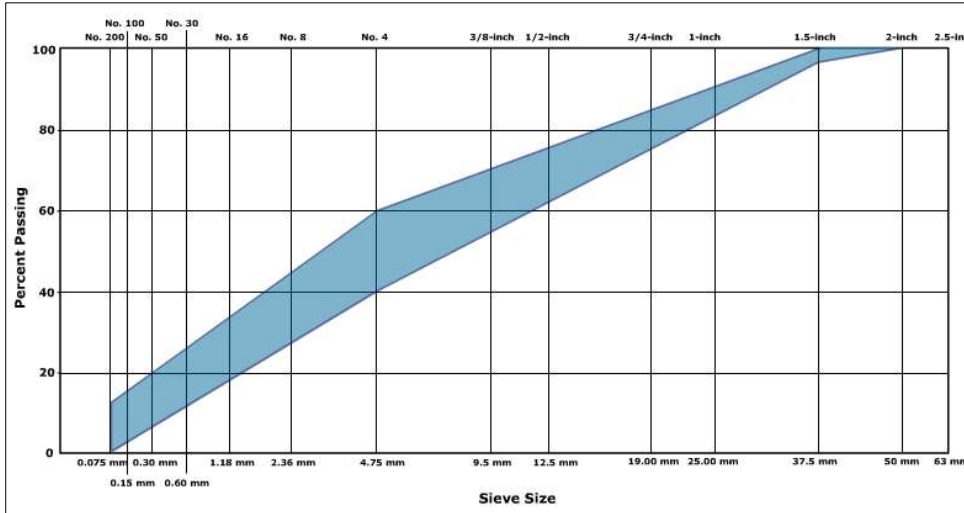


Image source: <https://pavementinteractive.org/reference-desk/materials/aggregate/gradation-and-size/>

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Gradation Specifications

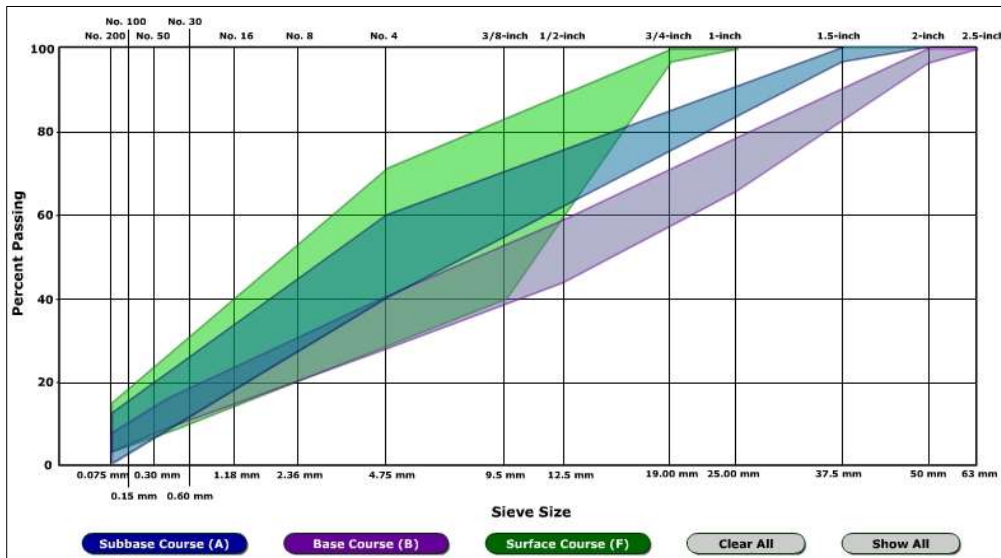


Image source: <https://pavementinteractive.org/reference-desk/materials/aggregate/gradation-and-size/>

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Aggregate

Aggregate blending

- ❑ A single aggregate source is generally **unlikely to meet gradation** requirements for Portland cement or asphalt concrete mixes
 - Thus, blending of aggregates from two or more sources would be required to satisfy the specifications.
- ❑ A **trial-and-error** process is generally used to determine the proportions
- ❑ The basic equation for blending is
 - $P_i = a \times A_i + B \times B_i + C \times C_i$;where
 - ❖ P_i = Percent blend materials passing sieve size i
 - ❖ A_i, B_i, C_i = Percent of aggregates from stockpiles A, B, C passing sieve size i
 - ❖ $a, b,$ and c = devimal fraction by weight of aggregates from stockpiles A, B, C used in the blend
 - * $a + b + c = 1.0$



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Aggregate blending

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Example -3



- Determine a blend of the two aggregates shown in the table below, which will meet the specifications

Sieve	12.5 mm (1/2 in.)	9.5 mm (3/8 in.)	4.75 mm (No. 4)	2.00 mm (No. 10)	0.425 mm (No. 40)	0.180 mm (No. 80)	0.075 mm (No. 200)
Specification	100	95–100	70–85	55–70	20–40	10–20	4–8
Target gradation	100	98	77.5	62.5	30	15	6
% Passing Agg. A (A_i)	100	100	98	90	71	42	19
% Passing Agg. B (B_i)	100	94	70	49	14	2	1

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Example -3

Solution



Sieve	12.5 mm (1/2 in.)	9.5 mm (3/8 in.)	4.75 mm (No. 4)	2.00 mm (No. 10)	0.425 mm (No. 40)	0.180 mm (No. 80)	0.075 mm (No. 200)
Specification	100	95–100	70–85	55–70	20–40	10–20	4–8
Target gradation	100	98	77.5	62.5	30	15	6
% Passing Agg. A (A_i)	100	100	98	90	71	42	19
% Passing Agg. B (B_i)	100	94	70	49	14	2	1
30% A_i ($a.A_i$)	30	30	29.4	27	21.3	12.6	5.7
70% B_i ($b.B_i$)	70	65.8	49	34.3	9.8	1.4	0.7
Blend (P_i)	100	96	78	61	31	14	6.4

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Aggregate classifications by size

Local classification



العدسية



الفولية



الجوزية



رمل



الناعمة (السمنية)



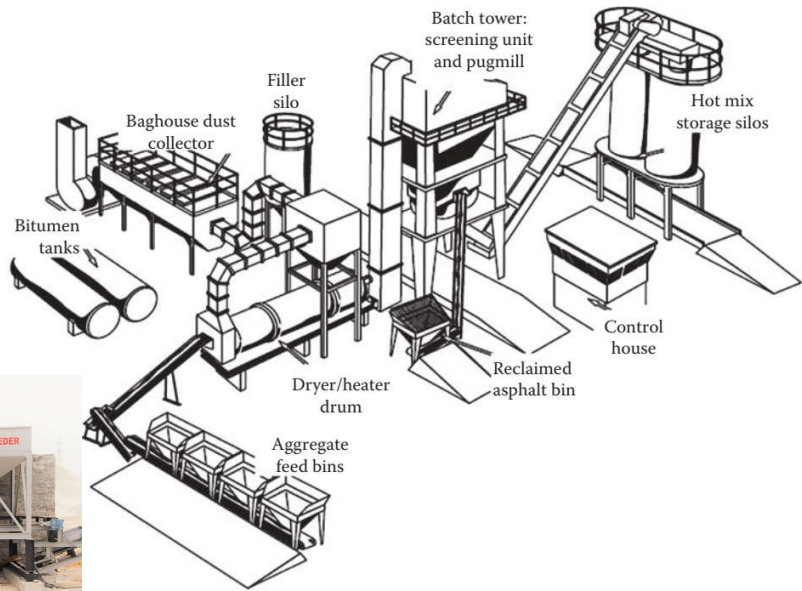
الحمصية

imgazr source: <https://www.slideserve.com/awel/4803086>

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HMA Manufacturing

The batch plant

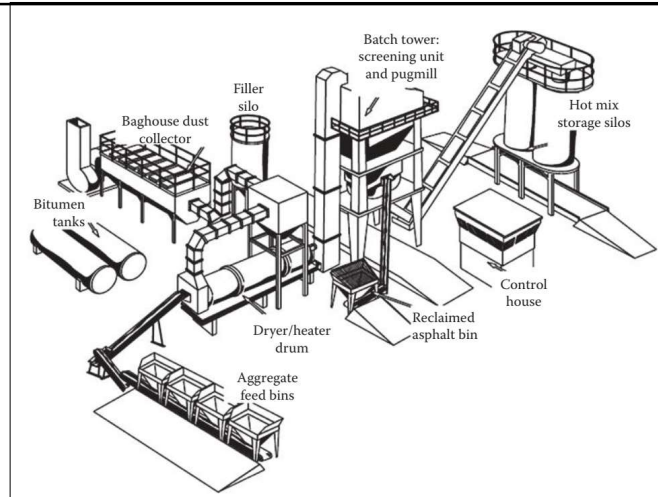
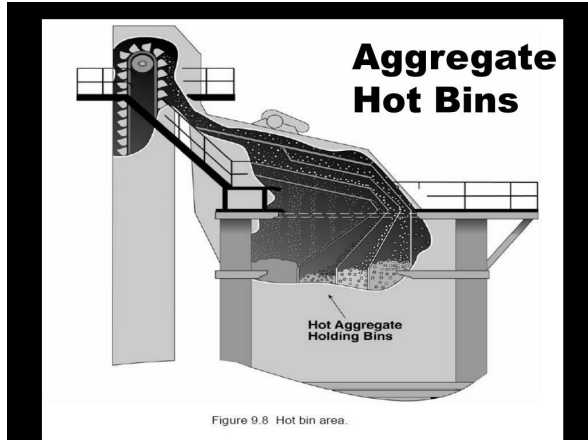


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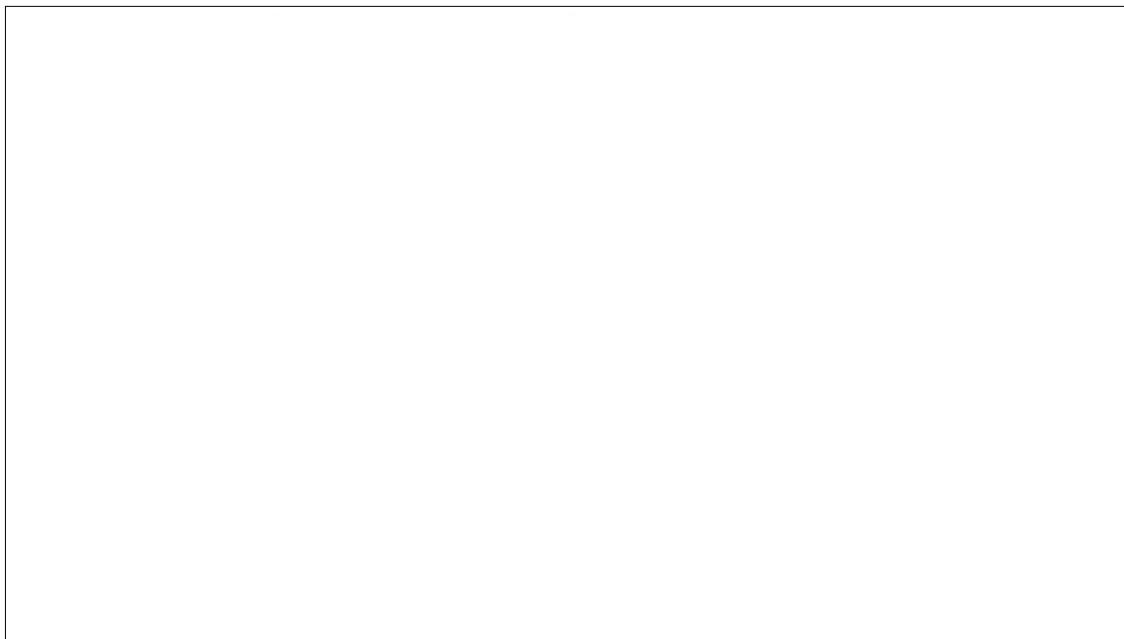
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HMA Manufacturing

The batch plant



HMA Manufacturing



Aggregate class

Local classification



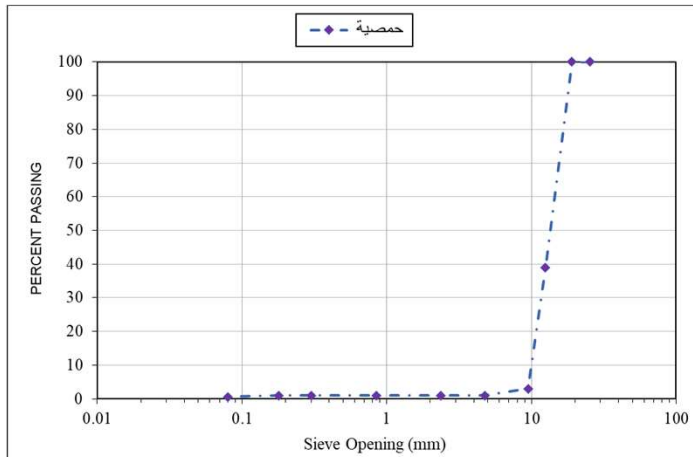
image source: https://www.slideserve.com/ave/4803086

Aggregate Identification		Coarse Agg.	Medium Agg.	Medium-Fine Agg.	Fine Agg.
		Limestone Aggregate			
		حصية	عذسية	حصية	ناعية
		زكام حجري			
Test Name		Test Result			
- Sieve Analysis: -		% Passing by Weight			
Sieve Number (Size, mm):	1" (25.4)	100	100	100	100
	3/4" (19.0)	100	100	100	100
	1/2" (12.7)	39	98	100	100
	3/8" (9.50)	3	32	100	100
	No. 4 (4.75)	1	4	18	100
	No. 8 (2.36)	1	3	4	66
	No. 20 (0.85)	1	3	4	38
	No. 50 (0.30)	1	3	4	24
	No. 80 (0.18)	1	3	3	20
No. 200 (0.075)	0.6	2.6	3.2	16	

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Aggregate classifications by size

Local classification

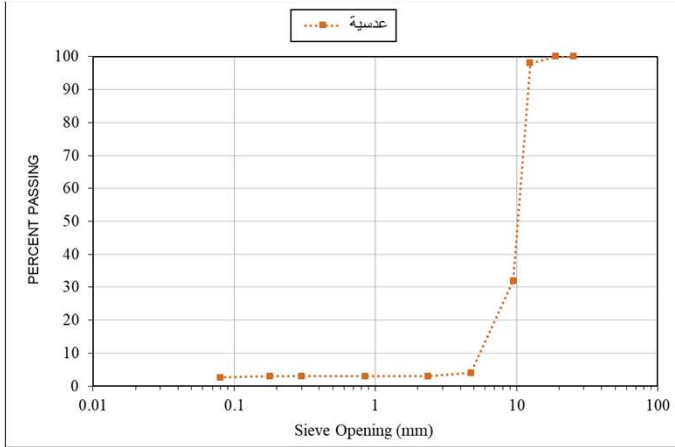


Test Name		Coarse Agg.	
		حصية	
		- Sieve Analysis: -	
		Sieve Number (Size, mm):	1" (25.4)
3/4" (19.0)	100		
1/2" (12.7)	39		
3/8" (9.50)	3		
No. 4 (4.75)	1		
No. 8 (2.36)	1		
No. 20 (0.85)	1		
No. 50 (0.30)	1		
No. 80 (0.18)	1		
No. 200 (0.075)	0.6		

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Aggregate classifications by size

Local classification



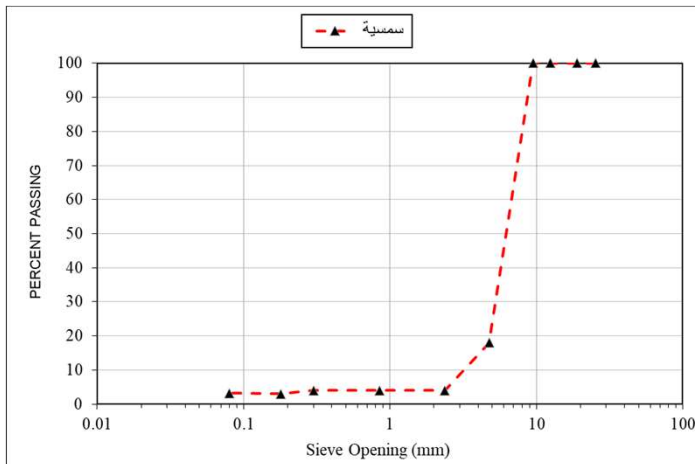
Medium Agg.	عسقية
Limestone	حجر

Test Name	Test Result
- Sieve Analysis: -	% Passing
1" (25.4)	100
3/4" (19.0)	100
1/2" (12.7)	98
3/8" (9.50)	32
No. 4 (4.75)	4
No. 8 (2.36)	3
No. 20 (0.85)	3
No. 50 (0.30)	3
No. 80 (0.18)	3
No. 200 (0.075)	2.6

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Aggregate classifications by size

Local classification



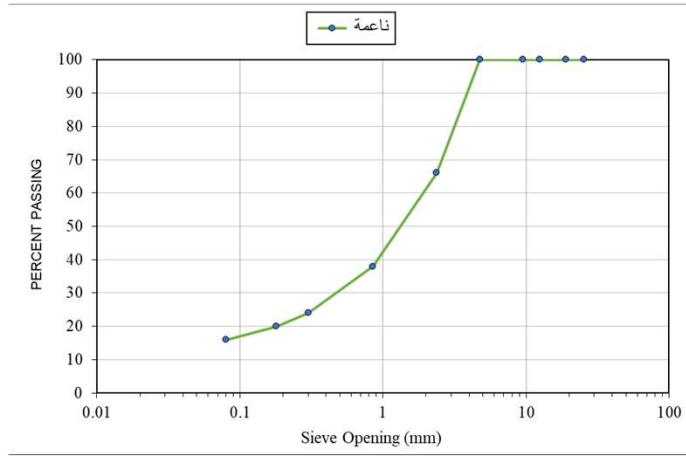
Medium-Fine Agg.	سمسية
Aggregate	حجر

Test Name	Result
- Sieve Analysis: -	by Weight
1" (25.4)	100
3/4" (19.0)	100
1/2" (12.7)	100
3/8" (9.50)	100
No. 4 (4.75)	18
No. 8 (2.36)	4
No. 20 (0.85)	4
No. 50 (0.30)	4
No. 80 (0.18)	3
No. 200 (0.075)	3.2

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Aggregate classifications by size

Local classification



Test Name		
- Sieve Analysis: -		
Sieve Number (Size, mm) :	1" (25.4)	100
	3/4" (19.0)	100
	1/2" (12.7)	100
	3/8" (9.50)	100
	No. 4 (4.75)	100
	No. 8 (2.36)	66
	No. 20 (0.85)	38
	No. 50 (0.30)	24
	No. 80 (0.18)	20
	No. 200 (0.075)	16

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Sieve No. (Size, mm)	Weight		
	Specification Limits		
1" (25)	100		
3/4" (19)	90	-	100
1/2" (12.5)	71	-	90
3/8" (9.5)	56	-	80
No. 4 (4.75)	35	-	56
No. 8 (2.36)	23	-	38
No. 20 (0.850)	13	-	27
No. 50 (0.300)	5	-	17
No. 80 (0.180)	4	-	14
No. 200 (0.075)	2	-	8



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HMA Mix Design

Asphalt Mixtures Volumetric

Specific Gravity for Aggregates

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Aggregate Specific Gravity

- ❑ Due to permeable voids in aggregates, three types of S. G. are defined
 - *Apparent (G_{sa})*
 - ❖ the ratio of the **oven-dry mass** of a unit volume of aggregate (**including only the impermeable void volumes**) to the mass of the same volume of water
 - *Aggregate Bulk Specific Gravity (G_{sb})*
 - ❖ the ratio of the **oven-dry mass** of a unit volume of aggregate (**including both the impermeable and water-permeable void volumes**) to the mass of the same volume of water
 - *Effective (G_{se})*
 - ❖ The ratio of the **oven-dry mass** of a unit volume of aggregate (**including both the impermeable void volumes and the water permeable voids not filled with absorbed asphalt**) to the mass of the same volume of water

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Aggregate Specific Gravity

Terms

Outside Surface Profile of Aggregate

Aggregate

Asphalt Binder

Impermeable Void

Permeable Void Portion Filled with Asphalt Binder (Absorbed Asphalt)

Permeable Void Portion NOT Filled with Asphalt Binder

Image source: <https://pavementinteractive.org/reference-desk/testing/aggregate-tests/coarse-aggregate-specific-gravity/>

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Aggregate Specific Gravity

Outside Surface Profile of Aggregate

Aggregate

Asphalt Binder

Impermeable Void

Permeable Void Portion Filled with Asphalt Binder (Absorbed Asphalt)

Permeable Void Portion NOT Filled with Asphalt Binder

V_s

V_{pp}

V_{ap}

$(V_{pp} - V_{ap})$

- V_s : Volume of solids
- V_{pp} : Volume of water permeable pores
- V_{ap} : Volume of pores absorbing asphalt
- $V_{pp} - V_{ap}$: Volume of water permeable pores not filled with asphalt

Image source: <https://pavementinteractive.org/reference-desk/testing/aggregate-tests/coarse-aggregate-specific-gravity/>

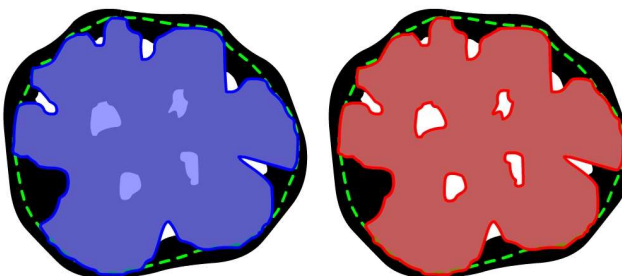
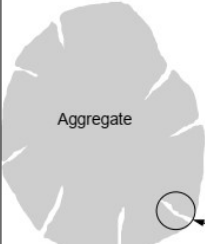
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Aggregate Specific Gravity

Apparent Specific Gravity, G_{sa}

- The ratio of the **mass in air** of a unit volume of an impermeable material at a stated temperature to the mass in air of equal density of an equal volume of gas-free distilled water at a stated temperature

Volumes Considered	Masses Considered
Aggregate particle	Aggregate particle (oven dry condition)

$$G_{sa} = \frac{\text{Dry Mass}}{\text{App Vol}} / 1.000 \text{ g/cm}^3$$

Apparent Volume = volume of solid aggr particle

Apparent volume does not include volume of surface pores

$$G_{sa} = \frac{W_s}{V_s \gamma_w} = \frac{M_s}{V_s \rho_w}$$

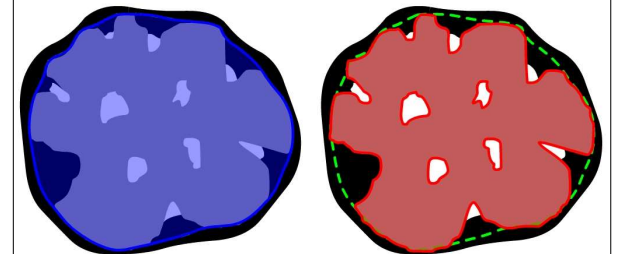
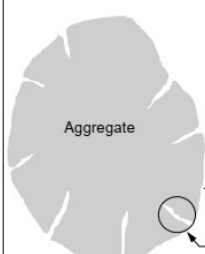
Image source: <https://pavementinteractive.org/reference-desk/testing/aggregate-tests/coarse-aggregate-specific-gravity/>

Aggregate Specific Gravity

Bulk Specific Gravity, G_{sb}

- The ratio of the **mass in air** of a unit volume of a permeable material to the mass in air of equal density of an equal volume of gas-free distilled water at a stated temperature.

Volumes Considered	Masses Considered
Aggregate particle + water permeable voids	Aggregate particle (oven dry condition)

$$G_{sb} = \frac{\text{Dry Mass}}{\text{Bulk Vol}} / 1.000 \text{ g/cm}^3$$

Bulk Volume = solid volume + water permeable voids

"SSD" Level

water permeable voids

$$G_{sb} = \frac{W_s}{(V_s + V_{pp}) \gamma_w} = \frac{M_s}{(V_s + V_{pp}) \rho_w}$$

Image source: <https://pavementinteractive.org/reference-desk/testing/aggregate-tests/coarse-aggregate-specific-gravity/>

Aggregate Specific Gravity

Coarse Aggregate Specific Gravity

- ❑ Determined in accordance with ASTM C 127
 - Wash **5 kg** of aggregate retained on **No. 4 sieve**.
 - Oven dry to a constant weight.
 - Soak in water for **24 hours**.
 - Decant water.
 - Use pre-dampened towel to get **SSD** condition, weigh and record (**B**)
 - Place the SSD sample in a wire basket, **submerge in water**, then the submerged weight is determined and recorded (**C**)
 - **Oven dry** the sample to a constant weight, weigh and record (**A**)

- ❑ A: Oven-dry wt. of agg.(g)
- ❑ B: SSD wt. of agg. (g)
- ❑ C: submerged wt. of SSD agg. In water (g)

$$G_{sa} = \frac{A}{A-C}$$

$$G_{sb} = \frac{A}{B-C}$$

$$G_{s, SSD} = \frac{B}{B-C}$$

$$\text{Absorption, \%} = \frac{(B-A) \times 100}{A}$$

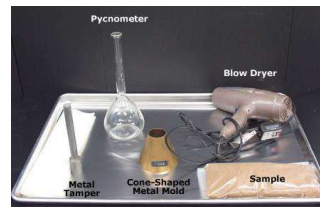
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Aggregate Specific Gravity

Fine Aggregate Specific Gravity

- ❑ Determined in accordance with ASTM C 128
 - Fill **flask with water** and record weight as (**B**)
 - Oven-dry **1000 g** of fine aggregate.
 - Soak in water for **24 hours**
 - Spread out and dry (warm air moving current) to **SSD?**
 - Add **500 g of SSD aggregate (D)** to pycnometer of **known volume** pre-filled with some water
 - Add more water and **agitate** until **air bubbles** have been **removed**
 - Fill to line and determine the **mass** of the **pycnometer, aggregate and water (C)**
 - Empty aggregate into pan and **dry to constant mass**
 - Determine **oven dry** mass (**A**)



$$G_{sa} = \frac{A}{B+A-C}$$

$$G_{sb} = \frac{A}{B+D-C}$$

$$G_{s, SSD} = \frac{D}{B+D-C}$$

$$\text{Absorption, \%} = \frac{(D-A) \times 100}{A}$$

- ❑ A: Oven-dry wt. of agg. (g)
- ❑ B: wt. of flask filled with water (to mark), (g)
- ❑ C: wt. of flask + SSD specimen + water (to mark), (g)
- ❑ D: SSD wt. (500 ± 10 g)

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HMA Mix Design

Asphalt Mixtures Volumetric

Specific Gravity for Aggregate Blend

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Aggregate Specific Gravity for blend

Aggregate Blend

- ❑ A single aggregate source is generally **unlikely to meet gradation** requirements for Portland cement or asphalt concrete mixes
 - Thus, blending of aggregates from two or more sources would be required to satisfy the specifications.
- ❑ A trial-and-error process is generally used to determine the proportions
- ❑ The basic equation for blending is
 - $P_i = a \times A_i + B \times B_i + C \times C_i$;where
 - ❖ P_i = Percent blend materials passing sieve size i
 - ❖ A_i, B_i, C_i = Percent of aggregates from stockpiles A, B, C passing sieve size i
 - ❖ a, b, and c = decimal fraction by weight of aggregates from stockpiles A, B, C used in the blend
 - * $a + b + c = 1.0$



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Aggregate Specific Gravity for blend



Composite G_{sb} for one stockpile

- ❑ For stockpiles that include both a **coarse and fine fraction**
 - One value must be determined for the stockpile.
- ❑ The average G_{sb} can be calculated as follows:

$$G_{sb} = \frac{P_{coarse} + P_{fine}}{\frac{P_{coarse}}{G_{coarse}} + \frac{P_{fine}}{G_{fine}}}, \text{ where}$$

- G_{sb} = bulk (dry) specific gravity of the aggregate
- P_{coarse} = percentage by weight retained on the No. 4 (4.75 mm) sieve
- P_{fine} = percentage by weight passing the No. 4 (4.75 mm) sieve
- G_{coarse} = bulk (dry) specific gravity of the coarse fraction
- G_{fine} = bulk (dry) specific gravity of the fine fraction

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Aggregate Specific Gravity for blend



Composite G_{sb} for one stockpile

- ❑ For stockpiles that include More than two aggregate sources
 - One value must be determined for the stockpile.
- ❑ The average G_{sb} can be calculated as follows:

$$G_{sb} = \frac{P_1 + P_2 + P_3 + \dots + P_n}{\frac{P_1}{G_1} + \frac{P_2}{G_2} + \frac{P_3}{G_3} + \dots + \frac{P_n}{G_n}}, \text{ where}$$

- G_{sb} = bulk (dry) specific gravity of the aggregate
- P_1, P_2, P_3, P_n = Percentages by weight of aggregates 1, 2, through n
- G_1, G_2, G_3, G_n = Bulk (dry) specific gravities of aggregates 1, 2, through n

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Aggregate Specific Gravity for blend



Absorption (A) for the aggregate blend

- ❑ The absorptiveness of aggregate is of significant interest to the mixture designer and specifier.
 - Absorption can be an indicator regarding aggregate quality along with increased binder demand.
- ❑ The binder absorption is typically 40 –80 percent of the water absorption rate
- ❑ The water absorption rate is calculated by the following equation as outlined in AASHTO T 85

$$\text{Absorption, \%} = \frac{B-A}{A} \times 100$$

- A = mass of the oven-dry test sample
- B = mass of the saturated surface-dry sample

- ❑ The average water absorption for the total aggregate blend as shown in AASHTO T 85 is calculated as follows

$$\text{Absorption \%} = \frac{P_1 \times A_1 + P_2 \times A_2 + \dots + P_n \times A_n}{100}, \text{ where}$$

- P_1, P_2, P_n = Percentages by weight of aggregates 1, 2, through n
- A_1, A_2, A_n = absorption of aggregates 1, 2, through n

Aggregate Identification	Coarse Agg. 1	Coarse Agg. 2	Medium Agg.	Medium-Fine Agg.	Fine Agg.
	(Basalt)			(Mixed)	
	حصية 1	حصية 2	عظمية	ممتزجة	ناعية
	(الركب)			(المزج)	
Test Name	Test Result				
- Sieve Analysis: -	% Passing by Weight				
Sieve Number (Size, mm):	1" (25)	100	100	100	100
	3/4" (19)	99	100	100	100
	1/2" (12.5)	1	54	100	100
	3/8" (9.5)	1	11	80	98
	No. 4 (4.75)	1	1	14	55
	No. 8 (2.36)	1	1	2	4
	No. 20 (0.850)	1	1	2	3
	No. 50 (0.300)	1	1	1	3
	No. 80 (0.180)	1	1	1	2
	No. 200 (0.075)	0.4	0.6	0.9	1.9
	- Specific Gravity	Bulk SG. (Oven Dry)	2.748	2.741	2.736
	Bulk SG. (SSD)	2.797	2.791	2.788	2.782
	Apparent SG.	2.890	2.886	2.887	2.903
- Water Absorption, %	1.8	1.8	1.9	2.3	2.6

And the obtained combined grading was as follows: -

Sieve No. (Size, mm)	% Passing by Weight	
	Combined Grading	Specification Limits
1" (25)	100	100
3/4" (19)	99.9	90 - 100
1/2" (12.5)	84.8	71 - 90
3/8" (9.5)	72.4	56 - 80
No. 4 (4.75)	47.1	35 - 56
No. 8 (2.36)	29.9	23 - 38
No. 20 (0.850)	16.8	13 - 27
No. 50 (0.300)	10.0	5 - 17
No. 80 (0.180)	7.8	4 - 14
No. 200 (0.075)	5.2	2 - 8

Hot Bin Components	Hot Bin Proportions, %
Coarse Agg. 1 (Hot Bin 1) حصية 1	7.0
Coarse Agg. 2 (Hot Bin 2) حصية 2	18.0
Medium Agg. (Hot Bin 3) عظمية	21.0
Medium-Fine Agg. (Hot Bin 4) ممتزجة	21.0
Fine Agg. (Hot Bin 5) ناعية	33.0
Total المجموع	100.0

Bulk Specific Gravity of Combined Aggregate (Gsb) =	2.723
Effective Specific Gravity of Combined Aggregate (Gse) =	2.779
Absorbed Asphalt by Weight of Aggregate (Pha) =	0.75%